Pearl Condominiums Philadelphia, PA



Picture Courtesy of Blackney Hayes

Joseph Lichman Jr.
Structural Option
Advisor: Dr. Hanagan
Penn State University



Presentation Outline

- Building Introduction
- Existing Structural System
- Goals of Redesign
- Structural Depth
- Breadth Studies
- Conclusions and Recommendations
- Acknowledgements



Building Introduction

- Location and Site: 9th and Arch Street
 Philadelphia, PA
- Size: 111,570 S.F.
- Number of stories: 6
- Completed: October 2007
- Building Use: Mixed Use Development Housing Including Retail on the Ground Floor and Apartments on the Upper Floors.
- *Total Cost.* \$22,646,674
- Design-Bid-Build



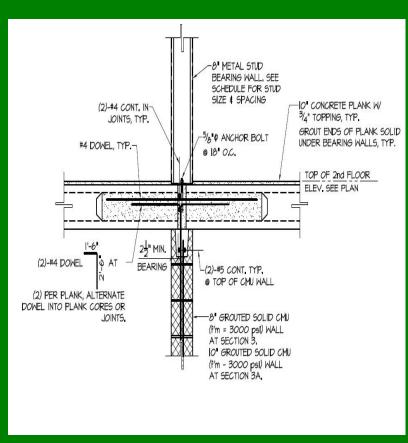
Building Introduction

- Project Team:
 - Owner Parkway Corporation
 - Architect Blackney Hayes Architects
 - Construction Manager JJ Deluca Company Inc.
 - Structural Engineer Pennoni Associates Inc.



Existing Structural System

- Current Structural System
 - Superstructure: Load Bearing Walls Composed of Metal Studs and Concrete Masonry Units
 - Roof System: Steel Joists and Metal Deck
 - Floor System: 10" Precast
 Concrete Plank with a ¾"
 Concrete Thick Topping
 - Foundation System: Grade Beams Bearing on Drilled Piers

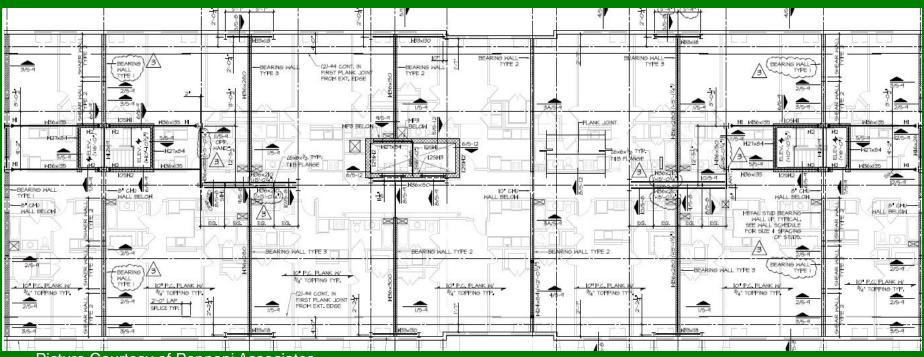


Picture Courtesy of Pennoni Associates



Existing Structural System

- Current Structural System
 - Second Floor acts as a Transfer Floor
 - Also Present are Large Steel Moment Frames



Picture Courtesy of Pennoni Associates



Existing Structural System

- Lateral System
 - Concrete Masonry Shear Walls
 - Metal Stud Shear Walls
 - Large Steel Moment Frames

LEMEL	VERTICAL CHORD REINFORCING (BOTH BNDS OF WALL)	VERTICAL REINFORCING (CENTERED IN CELL)	Pm (PSI)	CONTINUOUS HORIZONTAL REINFORCING IN BOND DEAM	REMARKS
7th - ROOF	2 - %	#5 € 48" O.C.	1500	#4 @ 48" O.C.	=
6th - 7th	2 - 46	#5 ● 32° O.C.	1500	#4 @ 48" O.C.	-
5th - 6th	2 - #7	#5 € 24° O.C.	1500	14 o 32° O.C.	-
3rd - 5th	4 - 87	#5 € 16' O.C.	1500	#5 ● 92" O.C.	-
2nd - 3nd	4 - 48	45 0 8° 0 C.	2000	#5 o 32° O.C.	5.
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VERTICAL	CHORD REINFORCIN (AMPLE 10-46 BARS	6 IS DETAILED AS IS EQUIVALENT TO IO* WALL THICK INTERNEDIATE VERTICAL REINFORGING	TOTAL NJ (2)-46 IN	STAIR TOWER CONTINUES CONTINUES HORIZONTAL REPPORTED IN TONO	
VERTICAL END. FOR EX	CHORD REINFORCIN (AMPLE IO-86 BARS) IOSM2 - VERTICAL CHORD REINFORCING	IO WALL THICK	TOTAL NJ (2)-46 IN KNESS	STAIR TOWER CONTINUOS HORIZONTAL	ALL AT EACH EN
VERTICAL END. FOR EX LEVEL	CHORD REINFORCIN (AMPLE IO-86 BARS) 105M2 - VERTICAL CHORD REINFORCING (BOTH ENDS OF HALL)	G IS DETAILED AS IS EQUIVALENT TO IO* WALL THICK VERTICAL REINFORCING (CONTROL OF COLUMN	(2)-46 IN KNESS Fm (PSI)	STAIR TOWER CONTINUOS HORIZONTAL REINFORMS IN DONO DEAM	REMARKS
VERTICAL END. FOR EX LEVEL	CHORD REINFORCIN (AMPLE IO-86 BARS 105M2 - VERTICAL CHORD REINFORCING (BOTH ENDS OF HALL) 2 - 95	G IS DETAILED AS IS EQUIVALENT TO IO* WALL THICK INTERNATION (SENTORICAL (SENTORICAL) (SENTORICAL) (SENTORICAL) (SENTORICAL) (SENTORICAL) (SENTORICAL) (SENTORICAL)	TOTAL NJ (2)-46 IN KNE96 Fm (P9I)	STAIR TOWER CONTINUOUS HORIZONTAL REINFORCING IN BOND BEAM #4 @ 48* O.C.	REMARKS
VERTICAL END. FOR EX LEVEL Teh - ROOF 6th - Teh	CHORD REINFORCIN (AMPLE IO-86 BARS) 105M2 - VERTICAL CHORD REINFORCING (BOTH ENDS OF HALL) 2 - 95 2 - 95	IS DETAILED AS IS EQUIVALENT TO MALL THICK INTERPORTATE ASSESSMENT OF THE ISSUE OF	TOTAL NJ (2)-46 IN KNESS Fm (PSI) 1500	STAIR TOWER STAIR TOWER CONTINUE CONTINUE	REMARKS
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	- BEA	RING WALL	₹ <u></u>	8° CN	411 8 0		- HI

	WALL TYPE I	8" MET	AL STUD WA	LL (13'-8" LEI	NGTH)	
LEVEL	COLUMN SECTION	BOOT TYPE	STRAP SIZE (EA. FACE)	CONNECTION AT EA. END	T, (KIPS)	ANCHOR BOLT
7th - ROOF	8000250-33	Α	4" - 54 mils	4 - #12	28	(2)-760
6th - 7th	8000250-54	В	4" - 54 mile	6 - #12	8.2 ^k	(2)-76"4
5th - 6th	(2) - 8000250-54	В	6" - 54 mils	12 - #12	19.0	(2)-V ₆ -9
3rd - 5th	(2) - 8000250-68	c	8" - 54 mile	16 - #12	16.7 ^K	(2)-7/6"4
2nd - 3nd	(2) - 8000250-47	D	8" - 54 mils	18 - #12	14.5 ^K	(2)-7/6°¢
-		-	141	12	-	
7th - ROOF	WALL TYPE 2 8000250-33	δ¹ ME1	TAL STUD WA	LL (9'-0" LE 4 - #12	NGTH) 3.2 ^K	(2)- ⁷ / ₆ °4
6th - 7th	8000250-54	В	4* - 54 mile	8 - #12	8.6 ^K	(2)-76-0
5th - 6th	(2) - 8006250-54	D	6* - 54 mlls	12 - #12	13.6 ^k	(2)-7/8°¢
3rd - 5th	(2) - 8000250-68	D	8* - 54 mile	l6 - #I2	175 ^k	(2)-7/5°4
2nd - 3nd	(2) - 8000250-97	D	8" - 54 mils	18 - #12	20.4 ^k	(2)-76"6
zna - sra						

Pictures Courtesy of Pennoni Associates



Goals Of Redesign

- Primary Goal
 - Eliminate the Dependency on Load Bearing Walls While Creating Minimal Change to Architectural Floor Plans
- Secondary Goal
 - Compare Cost and Schedule of New System to Existing Structure



- Redesign of Roof System
 - Added green roof (25 psf)
 - Longest Beam Span: 34'-9" (W14x61)
 - Longest Girder: 12'-1" (W14x34)
 - Columns: HSS 6x6x1/4





- Redesign of Floor System
 - Flex Frame
 - Combination of Precast Concrete Planks and Open-Web Dissymmetric Beams
 - Two Types of Dissymmetric Beams
 - » DB-8 and DB-9



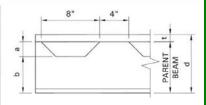
Pictures Courtesy of Girder-Slab



- Redesign of Floor System
 - Floor System
 - 8" Precast Concrete Planks with 2" Concrete Topping
 - Camber 1" for Span of 34'-9"
 - DB 9x46
 - Maximum Span: 13'
 - Tributary Width: 34'

D-Beam® Dimensions Table

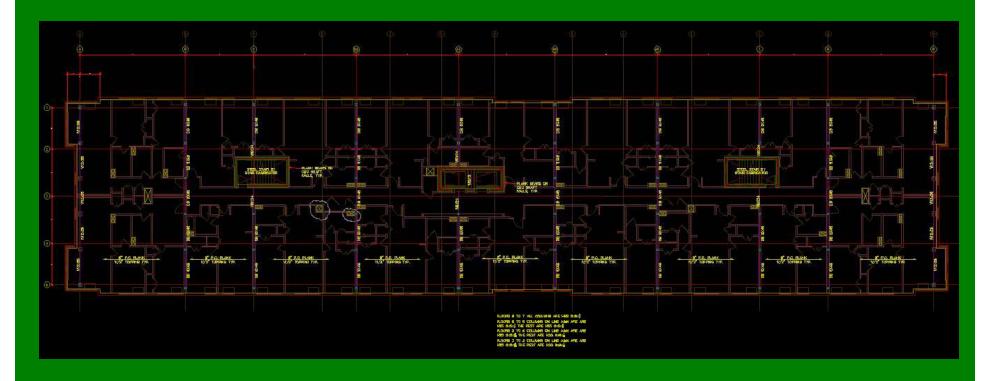
Designation	Web Included		Depth	Web	Parent Beam			2
	Weight	Avg. Area	d	Thickness t _w	Size	a	b	Top Bar w x t
	lb/ft	in²	in	in		in	in	in x in
DB 8 x 35	34.7	10.2	8	.340	W10 x 49	4	3	3 x 1
DB 8 x 37	36.7	10.8	8	.345	W12 x 53	2	5	3 x 1
DB 8 x 40	39.8	11.7	8	.340	W10 x 49	3	3.5	3 x 1.5
DB 8 x 42	41.8	12.3	8	.345	W12 x 53	1	5.5	3 x 1.5
DB 9 x 41	40.7	11.9	9.645	.375	W14 x 61	3.375	5.25	3 x 1
DB 9 x 46	45.8	13.4	9.645	.375	W14 x 61	2.375	5.75	3 x 1.5



Pictures Courtesy of Girder-Slab



Typical Floor Layout (Floor 3,5,6,7)





- Redesign of Lateral System
 - Change of CMU Shear Walls to Cast in Place Concrete Shear Walls
 - Result: 8" Concrete Walls with #5 @ 14" for Vertical and Horizontal Reinforcement
- Comparison of Foundation System
 - Drilled Piers: Cost \$18 Per Foot for 10" Diameter
 - Pile: Cost \$28 Per Foot for HP 10x42



- Conclusion
 - The Roof System
 - Beam and Girder System Decreases Overall Depth of System
 - Floor System
 - Flex Frame System Eliminates Reliance on Load Bearing Walls
 - Lateral System
 - Cast in Place Shear Walls Reduce the Overall Thickness of the Walls



Breadth Studies

- Construction Management
 - Goals of Breadth
 - Compare Cost Analysis For Existing and Redesigned Structures
 - Schedule Analysis of Redesign Structure vs. Existing Structure



Breadth Studies

- Cost Analysis
 - Comparison Considered the Second Floor Framing and Above
 - First Floor and Foundation Would be Similar for Both System
 - Existing Structure Cost: \$1,754,524
 - Proposed System Cost: \$1,760,136
- Schedule Comparison
 - Existing Structure Time: 3 months
 - Proposed System Time: 2 months 12 days



Breadth Studies

LEED Certification

Goals

- To Gain a minimum of a LEED Certification (26-32 points)
 - Sustainable Sites: Could Achieve as High as 9 out of 14 Possible Points
 - Material & Resources: Could Achieve as High as 10 out of 13 Possible Points



Conclusion

- Flex Frame System
 - Viable Alternative
 - Small Impact on Architectural Floor Plan
 - Proposed System Does Not Create a Substantial Change in Cost and Schedule
 - Cons
 - Limited By Possible Precast Concrete Planks (Only 8")
 - Would Redesign Second Floor Framing to Reduce Members Sizes



Acknowledgements

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- Pennoni Associate Inc.
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 - The entire AE faculty and staff



Questions?





